

Araştırma Makalesi – Research Article

The Effect of Over Consolidation Ratio on the Deformations of Tunnels Constructed in Clay Soils

Kil Zeminlerde İnşa Edilen Tünellerdeki Deformasyonlara Aşırı Konsolidasyon Oranının Etkisi

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ABSTRACT

With the globalization of the world, it has become necessary to create new routes for transportation due to the decrease in usage areas for construction and accelerated living conditions. For these reasons, some of the road and railway routes selected for transportation are passed through tunnels. In addition, with the increase in population in metropolitan cities, necessitated transportation by rail systems so tunnel constructions became a necessity here. However, these tunnels made on clay soils can cause major problems. Due to these problems, routes change, construction projects cannot be completed, and even during tunnel construction, tunneling devices become stuck in the tunnel and become unusable. In this study, the deformations of tunnels made in clay soils with different over consolidation ratios were investigated by using Plaxis2D program, which analyzes with the finite element method. The tunnel model was created using the New Austrian Tunneling Method (NATM) and the deformations in the clay soil were determined under static and dynamic loading conditions.

Keywords- Over Consolidation Ratio, Tunnel, Clay, Plaxis

ÖZ

Dünyanın küreselleşmesiyle günümüzde yapılaşma için kullanım alanları azalması ve hızlanan yaşam koşulları sebebiyle ulaşım için yeni güzergahlar oluşturulması zorunlu hale gelmiştir. Bu nedenlerle ulaşım için seçilen yol ve demiryolu güzergahlarının bir kısmı tünellerle geçilmektedir. Ayrıca büyükşehirlerde nüfusun artması buralarda raylı sistemlerle taşımacılığı zorunlu kılmış ve buralarda da tünel inşaatları gereklilik haline dönüşmüştür. Ancak kil zeminlerde yapılan bu tünellerde büyük problemler ortaya çıkabilmektedir. Bu problemlerden dolayı güzergahlar değişmekte, inşaat projeleri tamamlanamamakta, hatta tünel yapımı esnasında tünel açma cihazları tünel içerisinde sıkışarak kullanılamaz hale gelmektedir. Bu çalışmada sonlu elemanlar yöntemi ile analiz yapan Plaxis2D programı kullanarak, farklı aşırı konsolidasyon oranlarına sahip kil zeminlerde yapılan tünellerin deformasyonları incelenmiştir. Tünel modeli Yeni Avusturya Tünel Açma Yöntemi (NATM) kullanılarak oluşturulmuş ve statik ile dinamik yükleme koşullarında kil zemindeki deformasyonlar belirlenmiştir.

Anahtar Kelimeler- Aşırı Konsolidasyon Oranı, Tünel, Kil, Plaxis

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I. INTRODUCTION

Although accidents are less common in tunnels than in open lines, the effects of these accidents are more crucial and vital. Today, it has been faced with a decrease in usage areas for construction according to the globalization. For this reason, some of the road or railroad routes chosen for transportation are passed through underground tunnels.

Osmanoglu [1] studied the tunnels via finite element program Plaxis, which is one of the numerical analysis methods. With this study, the researcher investigated whether a building can be built on the soils where the tunnel exists and the deformations that occur in the tunnel lining. The results obtained were compared by applying soil improvement methods to keep the deformations within acceptable limits. Cortlever and Gutter [2] modeled the design of a set built according to the AuGeo method for the sections of the double railway line proposed in their study. Authors modeled the route, stating that soil improvement works are required to prevent long-term secondary consolidation settlement in the region. Cortlever and Gutter made the basic design according to the British Standard BS8006, which defines the load systems that should be included in the calculations. Finally, the proposed design method is verified with numerical analysis results of Plaxis software. Lahuta and Cihlarova [3] examined the performance analysis of an access road to the existing road network in Ostrava by mathematical modeling with Plaxis 2D. They also stated that this also helped them get feedback on the suitability of the selected measurement method and the effective distribution of the observed points on the structure. Turkoglu [4] analyzed the tunnel collapse in adverse soil conditions using two-dimensional numerical analysis. Turkoglu modeled the tunnel failure mechanism using the finite element method. It was concluded that unforeseen soil conditions would increase deformations significantly due to results. Selman [5] modeled the soil and tunnel excavation stages with the Plaxis program using the finite element method about the effects of excavation stageon the soil behavior of a large single switch tunnel. After the deformations and soil movements obtained, author tried to estimate the building damage with the Burland method. Sandström [6] examined the deformations of the tunnels constructed on London clay with the Plaxis program in his thesis. As a result of analysis, he emphasized that realistic results were obtained in the Plaxis program, but the correct selection of the parameters is very important.

In this study, the tunnel was modeled using the Plaxis2D program, which analyzes the deformations of the tunnels with the finite element method in clay soil conditions with different over consolidation rates. Plaxis is a finite element program, developed for the analysis of deformation, stability, and groundwater flow in geotechnical engineering [7]. In the tunnel model made with the New Austrian Tunneling Method (NATM), deformations in the clay soil under static and dynamic conditions were determined.

II. MATERIAL-METHOD

Within the scope of this study, deformations in the clay soil under static and dynamic conditions were determined in the tunnel made with the Austrian New Tunneling Method (NATM). The tunnel was modeled using the Plaxis 2D program according to following application stages. First of all, 3 different boreholes were drilled, and 3 different soil profiles were formed (Figure 1). The terrain model used in the study is presented in Figure 2.



Figure 1. Drilling

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Figure 2. Terrain model

Analyzes were carried out by using the Hoek-Brown method in tunnel construction [8]. The properties of analyzed clay soils are given in Table 1. The parameters of the soils used in the analyzes were created using the data obtained from the studies by Brinkgreve [9].

Table	1.	Clay	soil	properties
		Ciuj		properties

Soil layer	Soil	Young's Modulus	Poisson's Ratio	The Geological Strength Index (GSI)	Uniaxial compressive strength (σ_{ci})
1. Layer	Clay	0.200E6	0.2500	10	5.00E3
2. Layer	Clay stone	1.000E6	0.2500	40	25.00E3
3. Layer	Clay stone-Limestone	2.500E6	0.500	55	50.00E3

A. Static Conditions

After the formation of soil layers, the tunnel was created by excavating inside the soil and the interfaces of this tunnel were defined. The properties of the tunnel used in the study are given in Table 2.

Fable 2. The	properties	of the	e tunnel
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Parameter	Lining
Stiffness (EA)	$6.0 \times 10^6 \text{ kN/m}$
Flexural rigidity (EI)	$2.0 \times 10^4 \text{ kNm}^2/\text{m}$
Weight (<i>w</i>)	5.0 kN/m/m
Poisson's ratio (v)	0.15

Two different excavation procedures were carried out to prevent collapse during the tunnel excavation. In the first stage, the upper part of the tunnel was excavated and in the second stage this part was concreted. In the 3rd stage, the lower part of the tunnel was excavated, and this part was concreted in the 4th stage. After the land was divided into meshes to be used in the finite element method, the deformations at each stage were determined (Table 3) (Figure 3).

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Stages	Deformation (mm)
1 st stage (tunnel top excavation)	2.818
2 nd stage (tunnel top concreting)	4.070
3 rd stage (Tunnel sub-excavation)	4.644
4 th stage (tunnel sub-concreting)	4.592





Figure 3. The deformation of tunnel construction

B. Dynamic Conditions

After the completion of the tunnel construction, whose deformations were determined under static conditions, the acceleration of the earthquake which has a 5.4local magnitude (Ml) (Figure 4) was applied to the tunnel in the x and y axes (horizontal and vertical) and with the calculation method in the time-history analysis, the displacements of the tunnel were found under dynamic conditions (Table 4). The time-history acceleration graph is from Bentley Systems, the official company of Plaxis.



Figure 4. Time-history acceleration graph

Table 4. Deformations due to dynamic loading

Direction	Deformation (mm)
Х	74.27
Y	33.16







III. RESULTS AND DISCUSSION

In the scope of the study, the effect of over consolidation ratio on the tunnels that built in clay soils, was investigated. Ladd and Foott [10] investigated the change of normalized undrained shear strength with over consolidation ratio in their study using 5 different types of clays. The authors observed that the undrained shear strength increased as the over consolidation ratio increased (Figure 5). Drained shear strength in clays is determined as " $c_u = q_u / 2$ " [11]. Thus, in order to use different over consolidation ratios, analyzes were carried out using clay soils with uniaxial compressive strength (Table 5).



Figure 5. Relation of shear strength-over consolidation ratio

Table 5. Properties of soils used in analy	sis
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Analysis	Young's Modulus	Poisson's Ratio	The Geological Strength Index (GSI)	Uniaxial compressive strength (σ_{ci})
1 st analysis	1.000E6	0.2500	40	25.00E3
2 nd analysis	1.000E6	0.2500	40	5.00E3
3 rd analysis	1.000E6	0.2500	40	50.00E3

As a result of the analyzes made, the deformations that occur during tunnel construction are given in Table 6.

 Table 6. Deformations during tunnel construction

Analysis	1 st analysis	2 nd analysis	3 rd analysis
Stages		Deformation (mm)	
1 st stage (tunnel top excavation)	2.818	4.530	2.610
2 nd stage (tunnel top concreting)	4.070	6.427	4.018
3 rd stage (tunnel sub-excavation)	4.645	748.000	4.127
4 th stage (tunnel sub-concreting)	4.592	-	4.250

When the results of the soils used in the 1st and 3rd analyzes investigated, it was observed that deformations during tunnel construction decreased with the increase of uniaxial compressive strength due to the over consolidation ratio. In the clay soil used in the second analysis, due to the low over consolidation ratio, high amount of deformations occurred in the tunnel bottom excavation, which was the 3rd stage during the tunnel construction, and this caused the collapse of the soil (Figure 6).





Figure 6. Soil collapse during tunnel bottom excavation in the 2nd analysis

The deformations that occur under dynamic loading conditions in the 1st and 3rd analyzes, are given in Table 7. According to that, it was observed that the deformations decreased although there was not much difference in the case of dynamic loading with the increase of the over consolidation ratio. As a result of the dynamic analysis, it was observed that horizontal displacements are higher in tunnels compared to vertical displacements.

Table 7. Deformations under dynamic loading conditions in the 1st and 3rd analysis

Analysis	1 st analysis	3 rd analysis
Direction	Deforma	tion (mm)
Х	74.27	61.12
Y	33.16	22.83

IV. CONCLUSION

In this study, the tunnels were modeled using the Plaxis2D program, which analyzes the deformations of the tunnels with the finite element method in clay soil conditions with different over consolidation ratios. Thus, deformations that may occur under static and dynamic conditions in clay soils with different consolidation ratios were determined. As a result of the studies, when the analysis made in the soils were examined, it was observed that the deformations during the tunnel construction decreased with the increase of uniaxial compressive strength due to the over consolidation ratio. Although there was not much difference in the case of dynamic loading conditions, it was observed that with the increase of the over consolidation ratio, the deformations were decreased. In addition, as a result of dynamic analysis, it was observed that the horizontal displacements in the tunnels are higher than vertical displacements.

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