

Experimental Investigation of Scour Hole Geometry around Circular Bridge Piers

Ayşegül ÖZGENÇ AKSOY*¹, Mustafa DOĞAN¹

¹Dokuz Eylül Üniversitesi, Mühendislik Fakültesi, İnşaat Mühendisliği Bölümü, İzmir

Geliş tarihi: 19.07.2016

Kabul tarihi: 17.09.2016

Abstract

Failure of the bridges due to local scour around their piers can cause serious life losses and property damages and create significant environmental problems. In this experimental study, scour hole geometry around circular bridge piers were investigated. The experiments were carried out in a rectangular flume 80 cm wide, 18.6 m long and 75 cm high by using uniform sediment with a median diameter of 3.47 mm and geometric standard deviation of 1.39. The experiments were performed under steady state flow conditions, with four different constant discharges.

Keywords: Bridge pier, Clear-water scour

Dairesel Köprü Ayakları Etrafındaki Oyulma Çukuru Geometrisinin Deneysel Araştırılması

Öz

Köprü ayakları etrafındaki yerel oyulmalara bağlı oluşan sorunlar, köprülerin zarar görmesine ve buna bağlı ciddi çevresel etkilere sebep olmakta ve hatta can kayıpları yaşanabilmektedir. Sunulan bu deneysel çalışmada dairesel köprü ayakları etrafında oluşan oyulma çukuru geometrisi incelenmiştir. Deneyler, 80 cm genişlikli, 18,6 m uzunluklu ve 75 cm yükseklikli dikdörtgen kesitli bir kanalda, dane medyan çapı 3,47 mm ve geometrik standart sapması 1,39 olan üniform taban malzemesi ile gerçekleştirilmiştir. Dört farklı debi değeri ile kararlı akım şartlarında deneyler tamamlanmıştır.

Anahtar Kelimeler: Köprü ayağı, Temiz-su oyulması

* Sorumlu yazar (Corresponding author): Ayşegül ÖZGENÇ AKSOY, aysegul.ozgenc@deu.edu.tr

1. INTRODUCTION

Local scours around bridge piers influence their stabilities and can cause their failure. Therefore the estimation of the maximum possible scour depth around bridge piers is an important step in the design of the bridge pier foundations. It has been estimated that 60% of all bridge failures result from scour and other hydraulic related causes [1].

Flow parameters, bed material characteristics, pier geometry and time are affected the local scour process. Up to now various experimental researches have been carried out to understand the scour process [2-13].

In this study, the geometries of the scour holes obtained after each experiment were presented. The results can be helpful for future studies to estimate scour hole geometries.

2. EXPERIMENTAL SETUP AND EXPERIMENTAL RESULTS

The experiments were carried out in a rectangular flume 80 cm wide, 18.6 m long and 75 cm deep. Uniform granular sediment with median diameter d_{50} of 3.47 mm and geometric standard deviation of 1.39 was used in the flume. The experiments were performed under steady state flow conditions, with four different constant discharges by using 8 cm. The flow rates and approach flow depths were measured by using electromagnetic flow meter and ultrasonic level sensors, respectively. The slope of the flume was 0.6%. The experimental set-up is given in Figure 1.



Figure 1. Experimental set-up

The bridge piers were located at 11.5 m from the upstream end of the flume as shown in Figure 2.



Figure 2. The location of the bridge pier

The experiments were carried out using four different steady flow rates of 49 l/s, 55 l/s, 59 l/s, 65 l/s. The features of each experiment approach flow depth y , flow rate Q and flow intensity V/V_c is given in Table 1.

The critical velocity V_c was determined from the equation given below [14].

$$\frac{V}{u_{*c}} = 5.75 \log \left(5.53 \frac{y}{d_{50}} \right) \quad (1)$$

where u_{*c} is the critical shear velocity which can be calculated using the following relationship [6]:

$$u_{*c} = 0.0115 + 0.0125 d_{50}^{1.4} \quad (2)$$

for $0.1 \text{ mm} < d_{50} < 1 \text{ mm}$

$$u_{*c} = 0.0305 d_{50}^{0.5} - 0.0065 d_{50}^{-1} \quad (3)$$

for $1 \text{ mm} < d_{50} < 100 \text{ mm}$

In these relationships u_{*c} is in m/s and sediment size d_{50} is in mm.

Table 1. The details of the experiments

Exp. Ref.	D (cm)	Q (l/s)	y (cm)	V/V _c
1	8	49	21.0	0.37
2	8	55	21.9	0.39
3	8	59	22.8	0.40
4	8	65	23.3	0.43

The scour holes measured after each experiment along the centerline of the flume are shown in Figure 3.

The maximum scour depth d_s and the maximum length of the scour hole L_s measured after each experiment are given in Table 2.

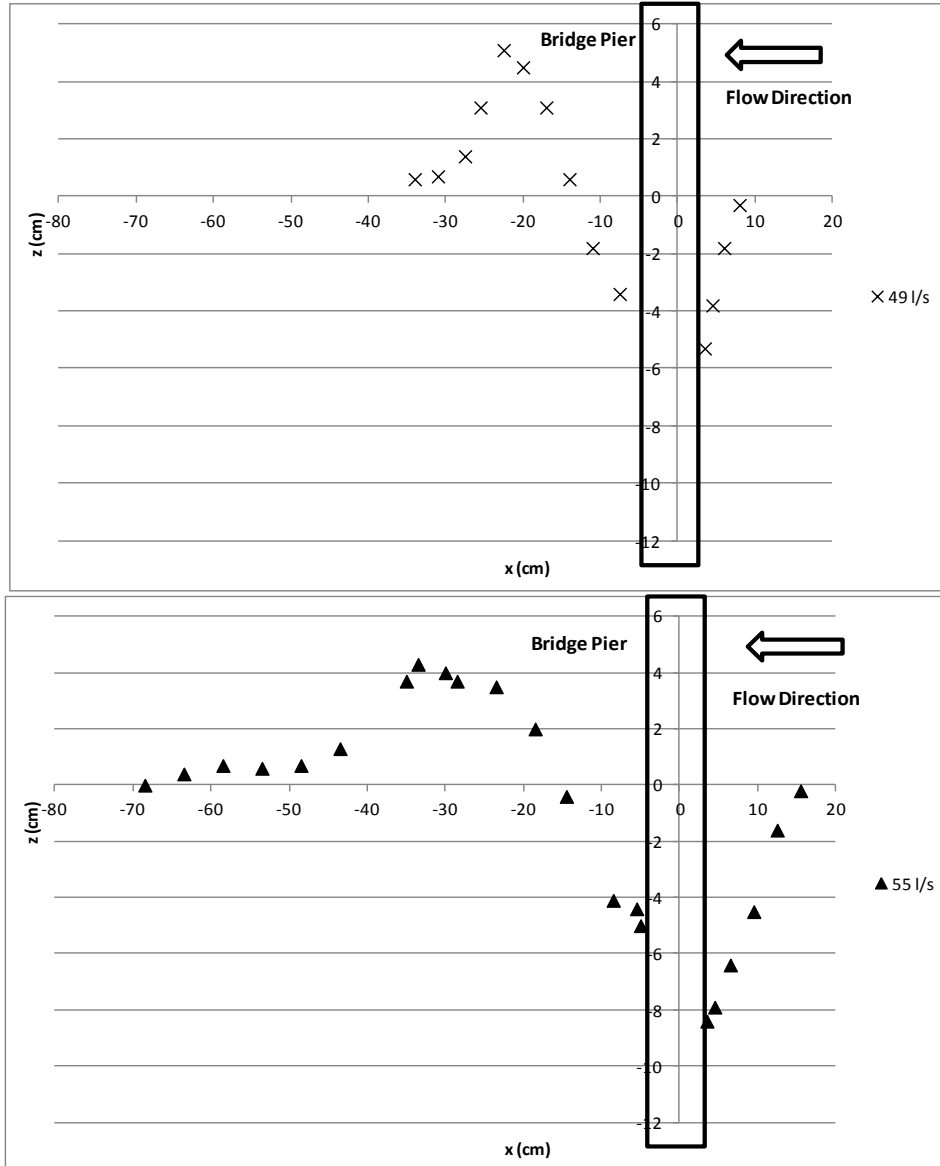


Figure 3. Measured scour hole along the centre line of the flume length for Q=49 l/s, Q=55 l/s, Q=59 l/s, Q= 65 l/s

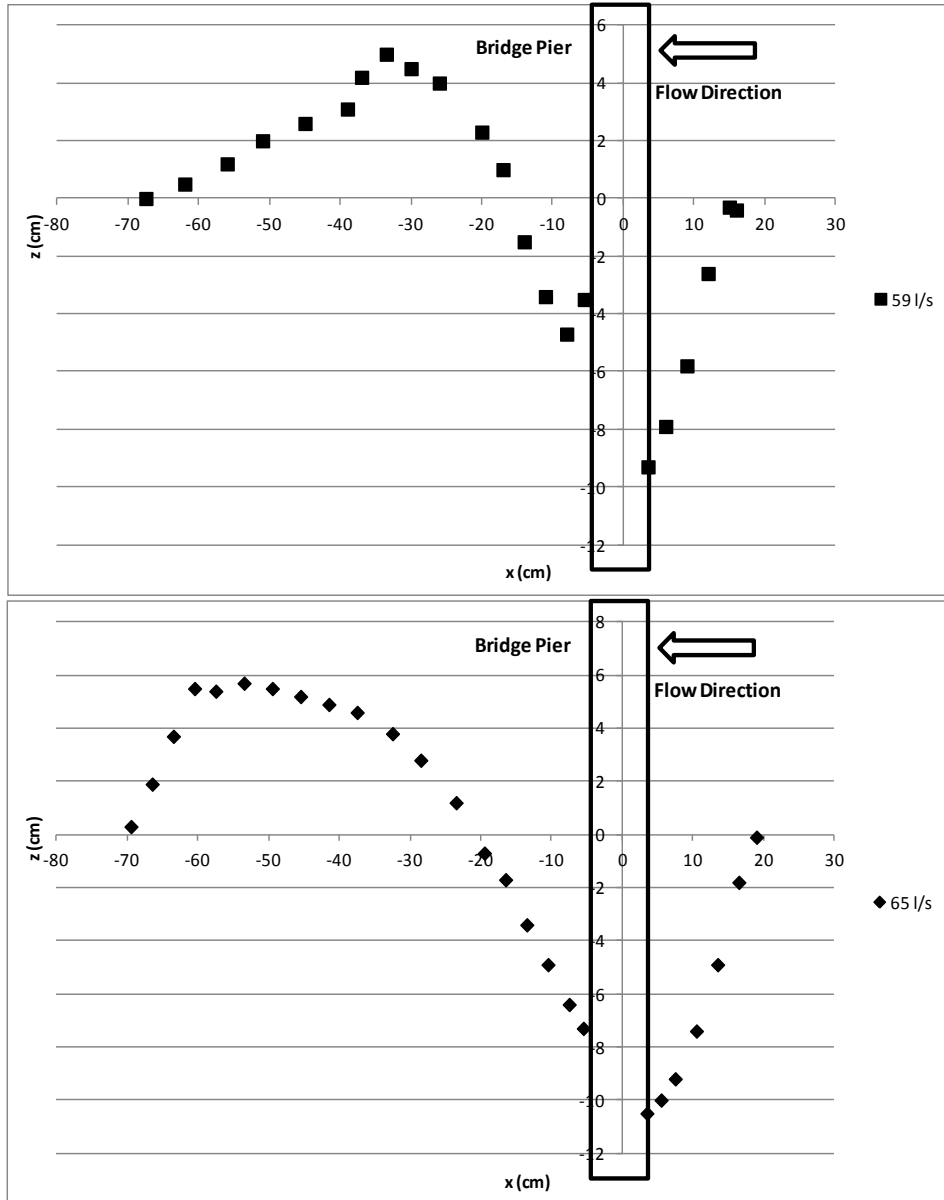


Figure 3. Continue

Table 2. The experimental results

Exp.Ref.	D (cm)	d_s (l/s)	L_s (cm)
1	8	5.3	20.0
2	8	8.4	30.5
3	8	9.3	32.0
4	8	10.5	39.0

The experimental results showed the following:

- Scour depth increased with flow velocity.
- The length of the deposit downstream of the pier and the length of the scour hole increased with flow velocity.

3. CONCLUSION

In this study the local scour around circular bridge piers was investigated experimentally. According to the experimental results, the scour depth increases with increasing flow velocity. It was observed that the length of the deposition downstream of the pier and the length of the scour hole also increase with flow velocity.

4. REFERENCES

1. Landers, M.N., 1992. Bridge Scour Data Management, In: ASCE, Hydraulic Engineering: Saving a Threatened Resource-in Search of Solutions, Proceedings, Hydraulic Engineering Sessions at Water Forum '92, 2-6 August, Baltimore, Maryland.
2. Laursen, E.M., 1958. Scour at Bridge Crossings, Bull. No.8, Iowa Hwy. Res. Board, Ames, Iowa.
3. Breusers, H.N.C., Nicollet, G., Shen, H.W., 1977. Local Scour Around Cylindrical Piers, Journal of Hydraulic Resources, 15(3): 211-252.
4. Gunyaktı, A., 1988. Köprü Ayakları Etrafında Oyulma Derinliğinin Grafik Yöntemle Tayini, Mühendislik ve Çevre Bilimleri Dergisi, TÜBİTAK, 12(1): 96-108.
5. Yanmaz, A.M., 1989. Time Dependent Analysis of Clear Water Scour Around Bridge Piers, Thesis Presented to Middle East Technical Univ., Ankara, Turkey, in Partial Fulfillment of the Requirements for the Degree of Doctor of Philosophy.
6. Melville, B.W., 1997. Pier and Abutment Scour: Integrated Approach, Journal of Hydraulic Engineering, 123(2): 125-136.
7. Richardson, E.V., Davis, S.R., 2001. Evaluating Scour at Bridges-Fourth Edition, Hydraulic Engineering Circular No.18, Federal Highway Administration Publication No. FHWA-NHI 01-001, Washington, DC, p. 376.
8. Oliveto, G., Hager, W.H., 2002. Temporal Evolution of Clear-water Pier and Abutment Scour, Journal of Hydraulic Engineering, 128(9): 811-820.
9. Mia, F., Nago, H., 2003. Design Method of Time-Dependent Local Scour at Circular Bridge Pier, Journal of Hydraulic Engineering, 129 (6): 420-427.
10. Chang, W.Y., Lai J.S., Yen, C.L., 2004. Evolution of Scour Depth at Circular Bridge Piers, Journal of Hydraulic Engineering, 130(9): 905-913.
11. Sheppard, D.M., Odeh, M., Glasser, T., 2004. Large Scale Clearwater Local Pier Scour Experiments, Journal of Hydraulic Engineering, 130(10): 957-963.
12. Kothyari U.C., Hager, W.H., Oliveto, G., 2007. Generalized Approach for Clear-water Scour at Bridge Foundation Elements, Journal of Hydraulic Engineering, 133(11): 1229-1240.
13. Sheppard, D.M., Demir, H., Melville, B., 2011. Scour at Wide Piers and Long Skewed Piers, National Cooperative Highway Research Program Rep. 682, Transportation Research Board, Washington, DC.
14. Melville, B.W., Sutherland, A.J., 1988. Design Method for Local Scour at Bridge Piers, Journal of Hydraulic Engineering, 114(10): 1210-1226.

